

## ON THE STABILITY OF VARLEY-DAY SOLUTIONS FOR HARMONIC MATERIALS

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**1. Introduction.** Varley and Day have considered the problem of determining deformations that can occur in a homogeneous, isotropic elastic body at constant pressure on the boundary and null Cauchy deviatoric stress and have found certain solutions to this problem [1] which possess constant principal stretches. Ericksen [2-3] has conjectured that the solutions obtained in [1] should be at best neutrally stable. This conjecture was proved for plane deformations by Adeleke [4].

In a recent paper [5] nonconformal plane deformations characterized by the requirement that the ratio of the principal stretches of the deformation is constant were shown to be Varley-Day type solutions (in the absence of body forces) for certain unconstrained isotropic elastic solids. By employing the stability criterion due to Beatty [6] the solutions in this class were also shown in [5] to be at best neutrally stable. The solutions found in [5] however are not the only plane nonconformal<sup>1</sup> Varley-Day solutions with nonconstant principal stretches. Such solutions may occur whenever the Baker-Ericksen (B-E) inequality [7, Sec. 51] is violated but the weakened B-E inequality is not [5].

One of the implications of the requirement that a Harmonic material [9] admits a regular state of uniaxial tension in plane strain is that the B-E inequality cannot be satisfied at all states of deformation [10]. Specifically, at deformations characterized by the condition that the sum of their principal stretches is a certain material-dependent constant the B-E inequality fails but the weakened B-E inequality does not [9, 10]. As it will become clear in the following this condition characterizes the class of plane Varley-Day solutions for Harmonic materials.<sup>2, 3</sup>

In this paper we show by means of examples that among the Varley-Day solutions for Harmonic materials which admit a regular state of uniaxial tension there are solutions with nonconstant principal stretches. Then, by using Beatty's stability

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<sup>1</sup>In the absence of body forces all conformal equilibrium solutions for unconstrained homogeneous isotropic elastic solids that satisfy the classical Pressure-Compression inequality [7, Sec. 51] must necessarily be homogeneous [8]. As such, they possess principal stretches that are both constant and equal.

<sup>2</sup>Throughout this paper the body forces are taken to be zero.

<sup>3</sup>There is an abuse of language here; the conformal (Varley-Day) solutions at which the B-E inequality is satisfied are not considered in this paper.

criterion for the infinitesimal stability of equilibrium configurations of unconstrained elastic solids subject to uniform hydrostatic loading everywhere on the boundary [6], we show that the Varley-Day solutions can be at best neutrally stable and, in particular, that the Varley-Day solutions with constant principal stretches are unstable. An illustrative example is considered in the last section of the paper.

**2. Preliminaries.** Consider the plane deformations described by a suitable smooth and invertible transformation

$$\mathbf{x} = \mathbf{x}(\mathbf{X}) \quad (2.1)$$

where  $\mathbf{X}$  and  $\mathbf{x}$  are points that belong, respectively, to the domains  $D$  and  $\bar{D}$  of the two-dimensional Euclidean Space  $R^2$ . Associated with such deformations is the deformation gradient, given by

$$\mathbf{F} = \frac{\partial \mathbf{x}}{\partial \mathbf{X}} \quad (2.2)$$

for which

$$\det \mathbf{F} > 0, \quad (2.3)$$

where  $\det(\cdot)$  stands for determinant. The deformation gradient can be uniquely represented in the form

$$\mathbf{F} = \mathbf{V}\mathbf{Q}, \quad (2.4)$$

where  $\mathbf{Q}$  is a proper orthogonal tensor and  $\mathbf{V}$  is a symmetric, positive-definite tensor often referred to as the left-stretch tensor. By the spectral theorem there is an orthogonal basis  $\{\mathbf{e}^1, \mathbf{e}^2\}$  such that

$$\mathbf{V} = \lambda_1 \mathbf{e}^1 \otimes \mathbf{e}^1 + \lambda_2 \mathbf{e}^2 \otimes \mathbf{e}^2, \quad (2.5)$$

where  $\otimes$  denotes the tensor product of two vectors. The scalars  $\lambda_1, \lambda_2$  are called principal stretches while the vectors  $\mathbf{e}^1, \mathbf{e}^2$  are the principal axes of strain. One set of invariants of  $\mathbf{V}$  is related to the principal stretches by the relations

$$\text{tr } \mathbf{V}^2 \equiv I = \lambda_1^2 + \lambda_2^2, \quad \det \mathbf{V} \equiv J = \lambda_1 \lambda_2, \quad (2.6)$$

where  $\text{tr}$  denotes the trace operator.

The plane part of the stress-deformation relation for a plane deformation of an isotropic homogeneous elastic solid can be written in the form

$$\mathbf{T} = \mathbf{t}(\mathbf{V}) = \varphi_0 \mathbf{1} + \varphi_1 \mathbf{V}^2. \quad (2.7)$$

In Eq. (2.7)  $\mathbf{1}$  is the identity tensor,  $\varphi_0$  and  $\varphi_1$  are scalar-valued functions of the invariants of  $\mathbf{V}$ , and  $\mathbf{T}$  is the Cauchy stress tensor. When a strain-energy function  $W$  exists, then

$$W = W(I, J) \equiv \widehat{W}(\lambda_1, \lambda_2), \quad (2.8)$$

and

$$\varphi_0 = \frac{\partial W}{\partial J}, \quad \varphi_1 = \frac{2}{J} \frac{\partial W}{\partial I}. \quad (2.9)$$

If we combine (2.5), (2.7), and (2.9) we find that

$$\mathbf{T} = t_1 \mathbf{e}^1 \otimes \mathbf{e}^1 + t_2 \mathbf{e}^2 \otimes \mathbf{e}^2, \quad (2.10)$$

where the principal stresses  $t_1, t_2$  are given by

$$t_i = \frac{\partial W}{\partial J} + \frac{2\lambda_i^2}{J} \frac{\partial W}{\partial I}, \quad i = 1, 2. \quad (2.11)$$

The equilibrium condition, in the absence of body forces, is

$$\operatorname{div} \mathbf{T} = \mathbf{0}, \quad \text{on } \bar{D}, \quad (2.12)$$

where  $\operatorname{div}(\cdot)$  denotes the divergence operator with respect to  $\mathbf{x}$ .

Harmonic materials are unconstrained elastic solids with a strain-energy density function in plane strain given by [9]

$$W(I, J) = 2\mu[H(Q) - J], \quad Q \equiv (I + 2J)^{1/2} = \lambda_1 + \lambda_2, \quad (2.13)$$

where  $\mu$  is a constant and  $H(\cdot) \in C^2(0, \infty)$ . Combining (2.7), (2.9), and (2.13) we find

$$\mathbf{T} = 2\mu \left\{ \left[ \frac{H'(Q)}{Q} - 1 \right] \mathbf{1} + \frac{H'(Q)}{QJ} \mathbf{V}^2 \right\}. \quad (2.14)$$

The requirement that both the strain-energy and the stress vanish in the undeformed configuration (i.e., for  $\lambda_1 = \lambda_2 = 1$ ) yields

$$H(2) = H'(2) = 1. \quad (2.15)$$

A material is strongly-elliptic at  $\mathbf{F}$  if [7, Sec. 44]<sup>4</sup>

$$A_{\alpha\beta\gamma\sigma} a_\alpha a_\gamma b_\beta b_\sigma > 0, \quad \alpha, \beta, \gamma, \sigma = 1, 2, \quad (2.16)$$

for every pair of nonzero vectors  $\mathbf{a}(a_\alpha)$ ,  $\mathbf{b}(b_\alpha)$ . Here  $A_{\alpha\beta\gamma\sigma}$  denote the components of the elasticity tensor  $\mathbf{A}$  which is the Fréchet derivative of the Piola stress<sup>5</sup>

$$\mathbf{S}(\mathbf{F}) = J(\varphi_0 \mathbf{F}^{-T} + \varphi_1 \mathbf{F}). \quad (2.17)$$

If Cartesian coordinates are assigned to  $D$  and  $\bar{D}$  the components of  $\mathbf{A}$  are given by [11]

$$\begin{aligned} A_{\alpha\beta\gamma\sigma} = & 4 \frac{\partial^2 W}{\partial I^2} F_{\alpha\beta} F_{\gamma\sigma} + 2J \frac{\partial^2 W}{\partial I \partial J} [F_{\beta\alpha}^{-1} F_{\gamma\sigma} + F_{\alpha\beta} F_{\sigma\gamma}^{-1}] + J^2 F_{\beta\alpha}^{-1} F_{\sigma\gamma}^{-1} \frac{\partial^2 W}{\partial J^2} \\ & + 2 \frac{\partial W}{\partial I} \delta_{\alpha\gamma} \delta_{\beta\sigma} + J \frac{\partial W}{\partial J} [F_{\beta\alpha}^{-1} F_{\sigma\gamma}^{-1} - F_{\sigma\alpha}^{-1} F_{\beta\gamma}^{-1}], \end{aligned} \quad (2.18)$$

where  $F_{\alpha\beta}$  denote the components of  $\mathbf{F}$ ,  $F_{\alpha\beta}^{-1}$  the components of  $\mathbf{F}^{-1}$ , and  $\delta_{\alpha\beta}$  the two-dimensional Kronecker symbol.

We shall suppose that strong ellipticity holds for infinitesimal deformations of Harmonic materials. As shown in [11], this is the case if and only if

$$\mu > 0, \quad H''(2) > 0. \quad (2.19)$$

<sup>4</sup>The summation convention over repeated indices is employed throughout the text.

<sup>5</sup> $\mathbf{F}^T$  is the transpose of  $\mathbf{F}$  and  $\mathbf{F}^{-T}$  is the inverse of the transpose of  $\mathbf{F}$ .

It then follows [10] that the strain-energy density (2.13) is positive, except in the undeformed state, if and only if

$$H(Q) > \frac{Q^2}{4}, \quad Q \in (0, \infty) - \{2\}. \quad (2.20)$$

From (2.6), (2.7), and (2.10) we find that at conformal deformations (with  $\lambda_1 = \lambda_2 \equiv \lambda$ )

$$t_1 = t_2 = \varphi_0(2J, J) + J\varphi_1(2J, J) \equiv t = \hat{t}(\lambda). \quad (2.21)$$

The classical Pressure-Compression inequality is the requirement that  $t$  be a strictly increasing function of  $\lambda$  [7, Sec. 51]. Slightly stronger than this is the requirement that

$$\frac{d\hat{t}}{d\lambda} > 0, \quad \lambda \in (0, \infty). \quad (2.22)$$

For Harmonic materials this condition becomes

$$QH''(Q) - H'(Q) > 0, \quad Q \in (0, \infty). \quad (2.23)$$

At a deformation with  $\lambda_1 \neq \lambda_2$  the stress-deformation relation (2.7) is said to satisfy the B-E inequality if [7, Sec. 51]

$$\varphi_1 > 0 \quad (2.24)$$

and the weakened B-E inequality if

$$\varphi_1 \geq 0. \quad (2.25)$$

At conformal deformations B-E inequality reduces to (2.25) [7, Sec. 51].

The inequality (2.24) is satisfied by the constitutive equation of Harmonic materials (2.14) if and only if

$$H'(Q) > 0, \quad Q \in (0, \infty). \quad (2.26)$$

However, we shall suppose that the Harmonic materials admit a regular state of uniaxial tension in plane strain [10]. As shown in [10], for this condition to be satisfied by the Harmonic materials which obey (2.15), (2.19), and (2.23) it is necessary and sufficient that there exists a number  $Q_0 \in (1, 2)$  such that

$$H'(Q_0) = 0, \quad H'(Q)/Q \rightarrow 1 \text{ as } Q \rightarrow \infty \text{ and } H''(Q) > 1 \text{ for } Q \in (Q_0, \infty). \quad (2.27)$$

A simple continuity argument shows that (2.27)<sub>3</sub> implies

$$H''(Q_0) \geq 1.^6 \quad (2.28)$$

An inspection of (2.14) and (2.12) now shows that the Varley-Day solutions for the class of Harmonic materials under consideration are characterized by the condition

$$\lambda_1 + \lambda_2 = Q_0. \quad (2.29)$$

<sup>6</sup>Note that at deformations with  $\lambda_1 + \lambda_2 = Q_0$  the Coleman-Noll (C-N) convexity condition [7, Sec. 87] merely requires  $H''(Q_0) > \frac{1}{2}$  [10].

**3. Nonconformal Varley-Day solutions with nonconstant principal stretches.** We start with the deformation

$$r = Rf(\Xi), \quad \theta = g(R, \Xi), \quad (3.1)$$

where  $(r, \theta)$  and  $(R, \Xi)$  are spatial and referential polar coordinates, respectively. The functions  $f(\cdot)$  and  $g(\cdot, \cdot)$  are to be determined such that the conditions (2.3) and (2.29) are satisfied.

The physical components of the deformation gradient corresponding to (3.1) are given by

$$\mathbf{F} = \begin{bmatrix} f & f_{,\Xi} \\ Rfg_{,R} & fg_{,\Xi} \end{bmatrix} \quad (3.2)$$

where,  $(\cdot)$  stands for differentiation with respect to  $(\cdot)$  (whether it is partial or total). From (2.4), (2.6), (3.2), and (2.29) we find the condition

$$I + 2J = f^2 + f_{,\Xi}^2 + R^2 f^2 g_{,R}^2 + f^2 g_{,\Xi}^2 + 2(f^2 g_{,\Xi} - Rff_{,\Xi}g_{,R}) = Q_0^2. \quad (3.3)$$

To obtain our first example we assume that  $g_{,R} = 0$  so that (3.3)<sub>2</sub> reduces to

$$f^2(1 + g_{,\Xi})^2 + f_{,\Xi}^2 = Q_0^2. \quad (3.4)$$

Taking  $f_{,\Xi} = k$ ,  $k = \text{const.}$ ,  $k \in (0, Q_0)$ , we obtain from (3.4)

$$f = k\Xi + k_1, \quad g = k^{-1} \sqrt{Q_0^2 - k^2 \ln(k\Xi + k_1)} - \Xi + k_2, \quad (3.5)$$

where  $k_1 > 0$  and  $k_2$  are constants of integration. Using (2.4), (2.6), (3.2), and (3.5) it is easy to show that

$$I - 2J = 4(k\Xi + k_1)^2 - 4\sqrt{Q_0^2 - k^2(k\Xi + k_1)} + Q_0^2 \neq \text{const.} \quad (3.6)$$

Since there exists a number  $\hat{k}$ , independent on  $Q_0$ , such that, for  $k_1 \in (0, \hat{k})$ , the condition (2.3) is satisfied it follows that (3.1) and (3.5) define a deformation which meets our requirements.

Next we suppose that, in addition to (3.3), the functions  $f$  and  $g$  satisfy

$$f_{,\Xi} = -Rfg_{,R}. \quad (3.7)$$

Combining (3.7) and (3.3) and taking

$$f = \alpha_0 \exp(\alpha_1 \Xi), \quad \alpha_0, \alpha_1 = \text{const.}, \quad (3.8)$$

in the resulting equation yields

$$g_{,\Xi} = \alpha_0^{-1} \sqrt{Q_0^2 \exp(-2\alpha_1 \Xi) - 4\alpha_0^2 \alpha_1^2} - 1, \quad (3.9)$$

where we assumed

$$4\alpha_0^2 \alpha_1^2 \leq Q_0^2 \exp(-4\alpha_1 \pi). \quad (3.10)$$

From (3.7), (3.8), and (3.9) we deduce

$$\begin{aligned} g(R, \Xi) = & -\alpha_0^{-1} \alpha_1^{-1} \sqrt{Q_0^2 \exp(-2\alpha_1 \Xi) - 4\alpha_0^2 \alpha_1^2} - \Xi - \alpha_1 \ln R \\ & + 2 \arctan \left[ 2^{-1} \alpha_0^{-1} \alpha_1^{-1} \sqrt{Q_0^2 \exp(-2\alpha_1 \Xi) - 4\alpha_0^2 \alpha_1^2} \right] + c, \end{aligned} \quad (3.11)$$

$c = \text{const.}$

As condition (2.3) is satisfied if and only if  $g_{,\Xi} > 0$ , we choose  $\alpha_0, \alpha_1$  so that

$$0 < \alpha_0 < 1, \quad 4\alpha_0^2\alpha_1^2 + \alpha_0 \leq \exp(-4\alpha_1\pi). \quad (3.12)$$

Noticing that

$$I - 2J = \alpha_0^2(1 - g_{,\Xi})^2 \exp(2\alpha_1\Xi) \neq \text{const.}, \quad (3.13)$$

we conclude that (3.8), (3.11), (3.1), and (3.12) define another deformation of the type sought here.

**4. Stability.** A criterion for the infinitesimal stability of equilibrium configurations of unconstrained homogeneous elastic solids subject to uniform hydrostatic loading  $p$  everywhere on the boundary has been obtained by Beatty in [6]. The specialization of this criterion to plane deformations of isotropic bodies takes the form

$$K \equiv \int_{\overline{D}} \{ \text{tr}[\mathbf{H}\mathbf{t}(\mathbf{V})\mathbf{H}^T] + \mathbf{H} \cdot \mathbf{C}(\mathbf{F})[\mathbf{H}] - p[2 \det \mathbf{E} + \text{tr}(\mathbf{R}\mathbf{R}^T)] \} d\overline{D} > 0 \quad (4.1)$$

for all nonzero plane infinitesimal displacements  $\mathbf{u}$ . Here  $\mathbf{H}(H_{\alpha\beta})$  is the gradient of  $\mathbf{u}$ ,  $\mathbf{E}$  and  $\mathbf{R}$  the symmetric and skew-symmetric parts of  $\mathbf{H}$ , respectively, and

$$\mathbf{H} \cdot \mathbf{C}(\mathbf{F})[\mathbf{H}] \equiv \left( \frac{1}{J} F_{\beta\nu} F_{\sigma\xi} A_{\gamma\xi\alpha\nu} - \delta_{\alpha\gamma} t_{\beta\sigma} \right) H_{\alpha\beta} H_{\gamma\sigma}, \quad (4.2)$$

where  $t_{\beta\sigma}$  denote the components of  $\mathbf{t}(\mathbf{V})$  and  $A_{\gamma\xi\alpha\nu}$  are given by (2.18). If  $K$  becomes negative for some  $\mathbf{u}$  then the equilibrium configuration is unstable while if  $K = 0$  for some  $\mathbf{u} \neq \mathbf{0}$  but nonnegative for all  $\mathbf{u}$  the equilibrium configuration is neutrally stable [6].

If  $\mathbf{F} = \mathbf{V}\mathbf{Q}$  is the deformation gradient corresponding to a Varley-Day solution for a Harmonic material it follows from (2.7) and (2.14) that

$$\mathbf{d}\mathbf{t}(\mathbf{V}) = p\mathbf{1} = -2\mu\mathbf{1}. \quad (4.3)$$

A straightforward but lengthy computation based on (2.18), (2.13), (4.2), (4.1), (4.3), and

$$\begin{aligned} \frac{\partial^2 \widehat{W}}{\partial \lambda_\alpha \partial \lambda_\beta} &= 4\lambda_\alpha \lambda_\beta \frac{\partial^2 W}{\partial I^2} + 2J \left( \frac{\lambda_\alpha}{\lambda_\beta} + \frac{\lambda_\beta}{\lambda_\alpha} \right) \frac{\partial^2 W}{\partial I \partial J} \\ &+ \frac{J^2}{\lambda_\alpha \lambda_\beta} \frac{\partial^2 W}{\partial J^2} + 2\delta_{\alpha\beta} \frac{\partial W}{\partial I} + (1 - \delta_{\alpha\beta}) \frac{\partial W}{\partial J} \end{aligned} \quad (4.4)$$

leads to<sup>7</sup>

$$K = 2\mu \int_{\overline{D}} \left\{ \left[ \frac{\lambda_1}{\lambda_2} H''(Q_0) - 1 \right] E_{11}^2 + 2H''(Q_0) E_{11} E_{22} + \left[ \frac{\lambda_2}{\lambda_1} H''(Q_0) - 1 \right] E_{22}^2 \right\} d\overline{D}. \quad (4.5)$$

Since there exists a nonzero displacement vector  $\mathbf{u}$  such that

$$E_{11} = E_{22} = 0 \quad (4.6)$$

the deformation with deformation gradient  $\mathbf{F}$  can be at best neutrally stable.

<sup>7</sup>  $F_{\alpha\beta}$  are the components of  $\mathbf{F}$  with respect to the basis  $\{\mathbf{e}^1, \mathbf{e}^2\}$ .

From (4.5) we deduce that a Varley-Day deformation with constant principal stretches is neutrally stable if and only if

$$[H''(Q_0)]^{-1} \leq \frac{\lambda_1}{\lambda_2} \leq H''(Q_0) \quad (4.7)$$

and

$$H''(Q_0) \left( \frac{\lambda_1}{\lambda_2} + \frac{\lambda_2}{\lambda_1} \right) \leq 1. \quad (4.8)$$

Since<sup>8</sup>

$$\{(\lambda_1, \lambda_2) \mid \lambda_1 + \lambda_2 = Q_0, H''(Q_0)(\lambda_1 \lambda_2^{-1} + \lambda_2 \lambda_1^{-1}) \leq 1, H''(Q_0) \geq 1\} = \emptyset, \quad (4.9)$$

the condition (4.8) is violated in any Varley-Day deformation. In the case of Varley-Day deformations with constant principal stretches this fact precludes neutral stability as clearly, given any such deformation, an infinitesimal displacement  $\mathbf{u}$  which renders  $K < 0$  must always exist.

**5. An example.** The Harmonic material with

$$H(Q) = \frac{1}{2}Q^2 + \frac{Q}{m-1} \left( \frac{2}{Q} \right)^m + \frac{1+m}{1-m}, \quad m \geq 0, \quad m \neq 1, \quad (5.1)$$

which was recently considered in [12], satisfies the restrictions (2.15), (2.19), (2.20), (2.23), and (2.27) with

$$Q_0 = 2^{m/(m+1)} \quad (5.2)$$

and

$$H''(Q_0) = 1 + m. \quad (5.3)$$

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<sup>8</sup>See the remark following (2.27) and footnote 6.