

— NOTES —

A NOTE ON SINGULARITIES IN A COSSERAT CONTINUUM*

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Abstract. This paper is concerned with the singularities that are due to concentrated couples in an infinite linear, elastic, isotropic Cosserat continuum. The solution to the problem of a concentrated couple, acting within an infinite region, may be obtained as a limiting case of the solution to the problem of body moments acting within a finite portion of the infinite medium. Alternatively, the solution to the problem of the concentrated couple can be constructed from the solution to the case of a concentrated force acting within an infinite body, by combining two double-forces with moments to form a center of rotation.

In this paper it is shown that in a Cosserat continuum the two above mentioned singular solutions to the case of a concentrated couple, acting within an infinite body, are not the same. By means of a specific linear combination of these two singular solutions it is possible to reconstruct the classical center of rotation, which is accompanied by an additional micro-rotation field. It is shown that there exists a limiting case in which the macro-displacements are eliminated altogether, resulting in a singular field of micro-rotations alone.

1. The equations of a linear, elastic, isotropic Cosserat continuum. In a Cosserat continuum [1]**, deformations are characterized by two kinematical variables: the displacement u_i and the independent, rigid, anti-symmetric micro-rotation $\psi_{[ij]}$. The quantities $\psi_{[ij]}$ describe a rigid rotation of some material "superstructural" property (e.g. the Cosserat triad or, alternatively, a "micro-structure").

Following Mindlin's formulation [2] of the linear, elastic case we define

$$\begin{aligned}\epsilon_{ij} &= \frac{1}{2}(u_{i,j} + u_{j,i}), \\ \gamma_{[ij]} &= \frac{1}{2}(u_{j,i} - u_{i,j}) - \psi_{[ij]}, \\ \kappa_{i[jk]} &= \psi_{[jk],i}.\end{aligned}\tag{1}$$

Then, for an isotropic, centrosymmetric medium the constitutive relations are

$$\begin{aligned}\tau_{ij} &= \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}, \\ \sigma_{[ij]} &= 2\beta \gamma_{[ij]}, \\ \mu_{i[jk]} &= \alpha_1(\kappa_{l[lj]} \delta_{ik} + \kappa_{l[kl]} \delta_{ij}) + 2\alpha_2 \kappa_{i[jk]} + \alpha_3(\kappa_{k[ij]} + \kappa_{j[ki]}),\end{aligned}\tag{2}$$

where τ_{ij} is the classical "Cauchy" stress, $\sigma_{[ij]}$ is the anti-symmetric part of Mindlin's relative stress, and $\mu_{i[jk]}$ is the Cosserat's couple-stress. The quantities α_i and β are

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**Numbers in square brackets indicate the reference listed at the end of this paper.

some material constants. The dimension of α_i differs from the dimension of the Lamé constants λ, μ by the square of length.

The equations of equilibrium read:

$$\begin{aligned}(\tau_{ij} + \sigma_{[ij]})_{,i} + f_i &= 0, \\ \mu_{i[jk],i} + \sigma_{[jk]} + \phi_{[jk]} &= 0.\end{aligned}\tag{3}$$

In (3) f_i and $\phi_{[ijk]}$ denote body force and body couple, respectively.

The boundary conditions, at a boundary with outward normal n_i , are

$$\begin{aligned}t_j &= n_i(\tau_{ij} + \sigma_{[ij]}), \\ T_{[ijk]} &= n_i\mu_{i[jk]}.\end{aligned}\tag{4}$$

Substituting (1) in (2), and then in (3) we obtain the kinematical equations of motion

$$\begin{aligned}(\lambda + \mu - \beta)u_{i,i} + (\mu + \beta)u_{i,ii} - 2\beta\psi_{[ij],i} + f_i &= 0, \\ (\alpha_1 + \alpha_3)(\psi_{[kij],ki} + \psi_{[ijk],ki}) + 2\alpha_2\psi_{[ij],kk} - 2\beta\psi_{[ij]} + \beta(u_{i,i} - u_{i,i}) + \phi_{[ij]} &= 0.\end{aligned}\tag{5}$$

Employing the "direct" notation, Eqs. (5) read

$$\begin{aligned}(\lambda + \mu - \beta)\nabla\nabla\cdot\mathbf{u} + (\mu + \beta)\nabla^2\mathbf{u} - 2\beta\nabla\cdot\psi^A + \mathbf{f} &= 0, \\ (\alpha_1 + \alpha_3)(\nabla\cdot\psi^A\nabla + \nabla\psi^A\cdot\nabla) + 2\alpha_2\nabla^2\psi^A - 2\beta\psi^A + \beta(\nabla\mathbf{u} - \mathbf{u}\nabla) - \frac{1}{2}\mathbf{I}\times\mathbf{c} &= 0.\end{aligned}\tag{6}$$

In (6) the quantities \mathbf{u} and \mathbf{f} are vectors with components u_i and f_i and ψ^A, ϕ^A and \mathbf{I} are dyadics with components $\psi_{[ij]}$, $\phi_{[ij]}$ and δ_{ij} . The quantity ϕ^A has been written in terms of a body couple vector \mathbf{c} ,

$$\phi^A = -\frac{1}{2}\mathbf{I}\times\mathbf{c}.$$

Mindlin has shown [2], that a complete solution of (6), or (5), can be expressed as

$$\begin{aligned}\mathbf{u} &= \nabla\times\mathbf{K} + (1 - l_3^2\nabla^2)(\mathbf{B} - l_1^2\nabla\nabla\cdot\mathbf{B}) - \frac{1}{2}(k_1 - l_3^2\nabla^2)\nabla[\mathbf{r}\cdot(1 - l_1^2\nabla^2)\mathbf{B} + B_0], \\ \psi^A &= -\frac{1}{4}\mathbf{I}\times[\nabla^2\nabla(\mathbf{r}\cdot\mathbf{K} + K_0) + 2\nabla\times\mathbf{B}]\end{aligned}\tag{7}$$

where $\mathbf{B}, B_0, \mathbf{K}$, and K_0 are stress-functions of the Boussinesq-Papkovich type. These functions satisfy the following relations:

$$\begin{aligned}\mu(1 - l_1^2\nabla^2)\nabla^2\mathbf{B} &= -\mathbf{f} - \frac{\mu + \beta}{2\beta}\nabla\times\mathbf{c}, \\ \mu\nabla^2B_0 &= \mathbf{r}\cdot\left[\mathbf{f} + \frac{\mu + \beta}{2\beta}\nabla\times\mathbf{c}\right], \\ 2\beta\nabla^2\mathbf{K} &= \mathbf{c},\end{aligned}\tag{8}$$

$$2\beta(1 - l_2^2\nabla^2)\nabla^2K_0 = 4l_2^2\nabla\cdot\mathbf{c} - \mathbf{r}\cdot(1 - l_2^2\nabla^2)\mathbf{c}$$

where, in (7) and (8)

$$\begin{aligned}k_1 &= \frac{\lambda + \mu}{\lambda + 2\mu}, \\ l_1^2 &= (2\alpha_2 - \alpha_1 - \alpha_3)\frac{\mu + \beta}{2\mu\beta}, \\ l_2^2 &= \frac{\alpha_2}{\beta}, \quad l_3^2 = \frac{2\alpha_2 - \alpha_1 - \alpha_3}{2\beta},\end{aligned}$$

2. The singular solutions for concentrated force and couple. Mindlin [2] has given the solution to the problems of a concentrated force and a concentrated couple acting within an infinite medium.

For a concentrated force \mathbf{P} , acting at the origin of a Cartesian coordinate system x, y, z the stress-functions are

$$\begin{aligned} \mathbf{B} &= \frac{\mathbf{P}}{4\pi\mu} g_1, \\ B_0 &= 0, \\ \mathbf{K} &= 0, \\ K_0 &= 0. \end{aligned} \tag{9}$$

For a concentrated moment \mathbf{C} , acting at the origin the stress functions are

$$\begin{aligned} \mathbf{B} &= -\frac{\mu + \beta}{8\pi\mu\beta} \mathbf{C} \times \nabla g_1, \\ B_0 &= 0, \\ \mathbf{K} &= -\frac{\mathbf{C}}{8\pi\beta r}, \\ K_0 &= -\frac{l_2^2}{4\pi\beta} \mathbf{C} \cdot \nabla g_2. \end{aligned} \tag{10}$$

In (9) and (10) $g_i = (1 - e^{-r/l_i})/r$.

Let $\mathbf{u}^{(1)}$ and $\psi^{A(1)}$ denote the kinematical field due to a concentrated force $P\mathbf{e}_z$ acting at the origin, and $\mathbf{u}^{(2)}$, $\psi^{A(2)}$ be the kinematical field that is due to a force $P\mathbf{e}_y$ acting at the origin.

Employing the fields $\mathbf{u}^{(1)}$, $\psi^{A(1)}$ and $\mathbf{u}^{(2)}$, $\psi^{A(2)}$, it is possible to construct a singular solution due to a "center of rotation about the axis of z " [3]. We let the forces $h^{-1}P\mathbf{e}_z$ and $-h^{-1}P\mathbf{e}_y$ act at the origin $(0, 0, 0)$, and the forces $-h^{-1}P\mathbf{e}_z$ and $h^{-1}P\mathbf{e}_y$ act at $(0, h, 0)$ and $(h, 0, 0)$, respectively, as shown in Fig. 1.

Passing to the limit as $h \rightarrow 0$ the resulting kinematical field is given by

$$\begin{aligned} \mathbf{u}^{(R)} &= \frac{\partial \mathbf{u}^{(1)}}{\partial y} - \frac{\partial \mathbf{u}^{(2)}}{\partial x}, \\ \psi^{A(R)} &= \frac{\partial \psi^{A(1)}}{\partial y} - \frac{\partial \psi^{A(2)}}{\partial x}. \end{aligned} \tag{11}$$

Computing the field $\mathbf{u}^{(R)}$ and $\psi^{A(R)}$ we obtain

$$\begin{aligned} \mathbf{u}^{(R)} &= -\frac{P}{4\pi\mu} (1 - l_3^2 \nabla^2) \mathbf{e}_z \times \nabla g_1, \\ \psi^{A(R)} &= \frac{P}{8\pi\mu} \mathbf{I} \times \nabla \times \mathbf{e}_z \times \nabla g_1. \end{aligned} \tag{12}$$

Expressions (12) may be obtained from (7) if we select

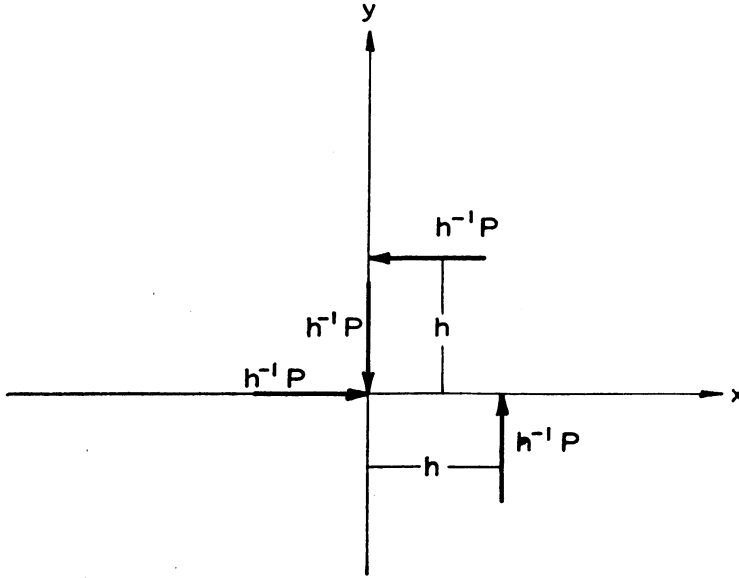


FIG. 1. The system of concentrated forces which yields, in the limit as $h \rightarrow 0$, a "center of rotation about the axis of z ."

$$\mathbf{B} = \mathbf{B}^{(R)} = -\frac{P}{4\pi\mu} \mathbf{e}_z \times \nabla g_1, \tag{13}$$

$$B_0^{(R)} = \mathbf{K}^{(R)} = K_0^{(R)} = 0.$$

For a concentrated moment directed about the z axis, at the origin $\mathbf{C} = C\mathbf{e}_z$, Eqs. (10) yield

$$\mathbf{B}^{(C)} = -\frac{\mu + \beta}{8\pi\mu\beta} C\mathbf{e}_z \times \nabla g_1 = \frac{\mu + \beta}{2\beta} \frac{C}{P} \mathbf{B}^{(R)},$$

$$B_0^{(C)} = 0, \tag{14}$$

$$\mathbf{K}^{(C)} = -\frac{C}{8\pi\beta r} \mathbf{e}_z,$$

$$K_0^{(C)} = -\frac{l_2^2 C}{4\pi\beta} \frac{\partial g_2}{\partial z}.$$

A comparison between (13) and (14) shows that the singularities due to the two kinds of concentrated couples are not the same. They differ by the stress functions \mathbf{K} and K_0 , given in (14). These functions yield a self-equilibrating kinematical field.

It may be worth noting that in the case of couple-stress theory [4], the singularity due to a concentrated couple and the singularity due to a "center of rotation" are the same.

3. Special cases. (a) Consider the following linear combination of the solutions to a concentrated couple $S^{(C)}$ and a center of rotation $S^{(R)}$.

$$S^{(L)} = -\frac{2P}{C} \frac{\beta}{\mu} S^{(C)} + \frac{\mu + \beta}{\mu} S^{(R)}. \tag{15}$$

Then

$$\begin{aligned}
 \mathbf{B}^{(L)} &= -\frac{2P}{C} \frac{\beta}{\mu} \mathbf{B}^{(C)} + \frac{\mu + \beta}{\mu} \mathbf{B}^{(R)} = 0, \\
 B_0^{(L)} &= 0, \\
 \mathbf{K}^{(L)} &= -\frac{2P}{C} \frac{\beta}{\mu} \mathbf{K}^{(C)} = \frac{P}{4\pi\mu} \frac{\mathbf{e}_z}{r}, \\
 K_0^{(L)} &= -\frac{2P}{C} \frac{\beta}{\mu} K_0^{(C)} = \frac{Pl_2^2}{2\pi\mu} \frac{\partial g_2}{\partial z}.
 \end{aligned} \tag{16}$$

The corresponding kinematical field is

$$\begin{aligned}
 \mathbf{u}^{(L)} &= \frac{P}{4\pi\mu} \nabla \times \frac{\mathbf{e}_z}{r}, \\
 \psi^{A(L)} &= \frac{P}{8\pi\mu} \mathbf{I} \times \nabla \frac{\partial g_2}{\partial z}.
 \end{aligned} \tag{17}$$

It is interesting to note that the expression for $\mathbf{u}^{(L)}$ has thus been made to agree with the classical result for a center of rotation about the axis of z . It does not depend on the "micro-parameters" of the Cosserat medium.

(b) Consider the limit of the solution $S^{(C)}$, for a concentrated moment about the z axis, as the ratio $\mu/\beta \rightarrow \infty$.

In this case the characteristic lengths $l_1^2 \rightarrow l_3^2$ and

$$(1 - l_3^2 \nabla^2) g_1 \rightarrow 1/r.$$

The kinematical fields become

$$\begin{aligned}
 \mathbf{u}^{(C)} &\rightarrow 0, \\
 \psi^{A(C)} &= -\frac{C}{16\pi\beta} \mathbf{I} \times \left[\frac{e^{-r/l_1}}{rl_1^2} \mathbf{e}_z + \nabla \left(\frac{\partial g_1}{\partial z} - \frac{\partial g_2}{\partial z} \right) \right].
 \end{aligned} \tag{18}$$

It is seen that for this limiting case the macro-displacements vanish, and the resulting singular field contains micro-rotations alone.

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REFERENCES

1. E. and F. Cosserat, *Théorie des corps déformables*, Hermann, Paris, 1909
2. R. D. Mindlin, *Stress functions for a Cosserat continuum*, Int. J. Solids Structures, **1**, 265-271 (1965)
3. A. E. H. Love, *A treatise on the mathematical theory of elasticity*, Dover, Fourth Ed. (Especially Sec. 132)
4. R. D. Mindlin and H. F. Tiersten, *Effects of couple-stresses in linear elasticity*, Arch. Rational Mech. Anal., **11**, 415-448 (1962)